



Macro-Micromechanical Properties of Sandy Pebble Soil of Different Coarse-Grained Content

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ABSTRACT

Sandy pebble stratum is a typical discrete particle unstable stratum, mainly consisting of sand and pebble. However, the effect of coarse-grained content on the stability of stratum is not clear. In this paper, taking the sandy pebble soil of different coarse-grained content in Chengdu City, Sichuan Province, China as the research object, a research on macro-mesomechanical properties of sandy pebble soil of different coarse-grained content was carried out using the method combining the indoor large-scale triaxial test of coarse-grained soil with the discrete element numerical triaxial test. The research results showed that the stress-strain curve of sandy pebble soil exhibited strain softening with the increase of coarse-grained content; when the confining pressure was the same, the stress peak increased and the strain when the peak was reached decreased gradually with the increase of coarse-grained content. It revealed the functional relationship between coarse-grained content and mechanical indexes of sandy pebble soil such as internal friction angle and cohesion, the internal friction angle and cohesion of sandy pebble soil linearly increased with the rise of coarse-grained material; it proposed the particle discrete element micro parameters of sandy pebble soil of different coarse-grained content, including contact modulus, friction coefficient, particle stiffness ratio, contact bond strength. The research results provided the theoretical support for the fine design and construction of sandy pebble stratum project.

Keywords: Sandy pebble soil; coarse-grained content; discrete element; macro-micromechanical properties.

Propiedades Macro y Micromecánicas de Suelos de Arena Guijosa con Diferente Clasificación Granulométrica

RESUMEN

La capa de arena guijosa es un manto típico de partículas inestables que consisten principalmente de arena y guijarros. Sin embargo, el efecto del contenido de grano grueso en la estabilidad de la capa no es claro. El área de investigación en este estudio es la ciudad de Chengdu, provincia de Sichuan, en China. Allí se tomaron muestras de diferente clasificación granulométrica para analizar las propiedades macro y mesomecánicas del suelo de arena guijosa a través del método combinado de la prueba triaxial a gran escala en laboratorio para suelos de grano grueso con la prueba triaxial numérica de elementos discretos. Los resultados de la investigación muestran que la curva del ensayo de tracción en suelos de arena guijosa presenta un aplacamiento en la presión con el incremento del contenido de grano grueso. Cuando la presión de confinamiento fue la misma, el pico de esfuerzo se incrementó y la presión empezó a caer gradualmente cuando se alcanzó el pico con el incremento del contenido de grano grueso. Esto revela la relación funcional entre el contenido de grano grueso y los indicadores mecánicos para suelos de arena guijosa como el ángulo de fricción interna y la cohesión; estos factores se incrementaron con el aumento del material de grano grueso. Esto sugiere los microparámetros de partículas de elementos discretos en este tipo de suelos, que incluyen el módulo de contacto, el coeficiente de fricción, la proporción de rigidez de partículas y la fuerza del vínculo de contacto. Los resultados de la investigación proporcionan el apoyo teórico para el diseño y construcción de nuevos proyectos sobre la capa arenosa y guijarrosa.

Palabras clave: Suelo arenoso y guijaroso; contenido de grano grueso; elementos discretos; propiedades macro y micromecánicas.

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INTRODUCTION

Sandy pebble soil is widely distributed in various regions of the world, and its mechanical properties are further complicated by the randomness of geometrical distribution of pebble and discreteness of particle. The sandy pebble stratum widely distributed in Chengdu City, Sichuan Province, China has a great impact on the construction safety of tunnel and underground engineering(Wei et al., 2008; OmaraShahrestan and OmaraShastani, 2017), so the study on strength characteristics and stability of sandy pebble soil has been paid increasing attention (Si et al., 1990; Hejazi et al., 2017). Guo Qingguo (1998)proposed 5 mm as the dividing line of fine and coarse particles, and that takes 30% and 70% of coarse-grained contents as the two research points. At the same time, many scholars believed that the physical and mechanical properties of sandy pebble soil of different coarse-grained content such as cohesion, internal friction angle and permeability had a great impact (Wei et al., 2008; Yasin et al., 2017).Most of them conducted studies on macromechanical properties of sandy pebble soil using the triaxial test and direct shear test but lacking micromechanical parameters of the composite structure of sand and pebble particles.

Discrete element method is a numerical simulation method for solving discontinuous media problems, and has received attention from many scholars and was developed since being proposed in the 1970s by Cundall (1997). Geng Li(2011)simulated the triaxial test process using PFC3D, and analyzed the effect of particle micro parameters on the strength of coarse-grained soil, however, it researched on only a gradation of soil, so the conclusions were not representative. Xu Xiaofeng (2013) et al established the calculation model of discrete element simulation of four kinds of gravelly soils of different coarse-grained content, and analyzed the effect mechanism of coarse-grained content on the macro-micromechanical properties of gravelly soil, however, the micro parameters obtained were only suitable for a confining pressure, so it was not accurate enough. Qi Yang (2015)et al simulated the uniaxial loading test of coarse-grained soil in various main stress directions under triaxial state by means of PFC3D, and analyzed the stress-strain characteristics of coarse-grained soil in complex stress state from the macro and micro points of view, believing that the complex stress state was the inducing factor for coarse-grained soil to produce various anisotropies; in addition, many scholars at home and abroad solved the problems of sandy pebble soil encountered in the engineering by using the discrete element theory (Qi et al., 2015; Ahmad et al., 2017).

In this paper, relying on the sandy pebble stratum in Chengdu City, Sichuan Province, China, the influencing law of coarse-grained content on the macromechanical properties of sandy pebble soil was studied using the large-scale triaxial test of coarse-grained soil, on this basis, a particle discrete element numerical triaxial test platform was established, and the micromechanical parameters of sandy pebble soil of different coarse-grained content was studied using the discrete particle element numerical triaxial test (Yasin et al., 2017). The research results provided essential reference and guidance for the fine design, construction, and maintenance of sandy pebble stratum project.

RESEARCH METHOD

TRIAXIAL TEST OF COARSE-GRAINED SOIL

TEST SCHEME

In this test, a stress type large-scale triaxial test apparatus was adopted, and its specifications included a diameter of 300 mm, a height of 600 mm, a maximum axial stress of 21 MPa, a maximum vertical load of 1500 kN, maximum confining pressure of 3.0 MPa, and maximum axial stroke of 300 mm. The shear strength indexes c and ϕ of coarse-grained soil with the particle size of not less than 60 mm can be measured, as shown in Figure 1.



Figure 1. Large-scale triaxial apparatus of coarse-grained soil.

SPECIMENS

In this test, the sandy pebble soil in Chengdu City, Sichuan Province, China was taken as the research object, for the oversized particles, the weighted average substitution was conducted using the sandy pebble soils with the maximum allowed particle size of 60 mm to 5 mm according to the content, namely equivalent substitution method (Yasin, 2017; Kudus et al., 2017). To obtain the shear strength index of sandy pebble soil of different coarse-grained content, a total of four groups of specimens with P5 (mass percentage of particles with particle size of greater than 5 mm) of 73%, 75%, 76.5% and 80% were adopted in this test, with density of 2.05 g/cm³, moisture content of 1.6%. Each group of specimens was exerted with confining pressures of 100 kPa, 200 kPa, and 300 kPa. The particle gradation of sandy pebble soil used in the test is shown in Table 1 and Figure 2.

Table 1. Composition of particle gradation of sandy pebble soil of different coarse-grained content.

Particle size (mm)	Gradation 1	Gradation 2	Gradation 3	Gradation 4
	Percentage of particle sizes (%)			
40<d<60	35.07	33.57	33.18	32.14
20<d<40	31.88	30.49	30.17	29.2
10<d<20	7.06	6.01	6.61	6.27
5<d<10	5.99	5.7	5.67	5.52
2<d<5	2.4	2.65	2.62	2.55
1<d<2	0.79	0.87	0.87	0.84
0.5<d<1	5.88	6.49	6.42	6.86
0.25<d<0.5	6.31	6.96	6.9	6.71
0.075<d<0.25	3.63	4.66	4.01	3.91
d<0.075	1	2.5	3.56	6

The triaxial specimens were divided into three layers for preparation, which allows realizing the uniform mixing of particles. The shearing with an axial strain rate of 1.5 mm/min was carried out using the test method of isotropic consolidation undrained shear until the failure of the specimen occurred (1999), and the specimens are shown in Figure 3.

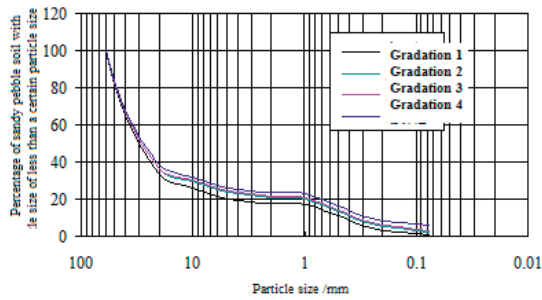


Figure 2. Gradation curve of sandy pebble soil.



Figure 3. Large-scale triaxial specimen of sandy pebble soil.

PARTICLE DISCRETE ELEMENT NUMERICAL TRIAXIAL TEST

ESTABLISHMENT OF NUMERICAL TRIAXIAL TEST PLATFORM

The test silo used in the numerical triaxial test of coarse-grained soil adopted a cylinder with diameter of 300 mm and height of 600 mm, its upper and lower planes were the loading plates with stiffness of greater than particle stiffness, the wall of the test silo adopted the flexible constraint, and its normal stiffness was smaller than normal stiffness of particles (Wun et al., 2017). In the effective area of the test silo, the sphere unit was adopted to replace the soil particle according to the particle gradation of sandy pebble soil tested in the test, the minimum unit diameter in the numerical triaxial test was $d_{min}=1$ mm, and the particles of large particle size were composed of small units and classified as a particle group (i.e., Clump unit) (Ridzuan et al., 2017). The contact bond model among particles was adopted, and the servo loading was adopted in the test to keep constant confining pressure. The model of the numerical triaxial test is shown in Figure 4.

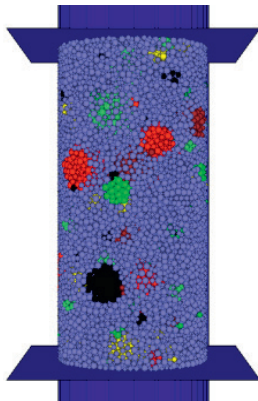


Figure 4. Triaxial numerical model.

METHOD FOR DETERMINING THE MICRO PARAMETERS OF DISCRETE ELEMENT

(1) Determination of particle contact stiffness E_c and ratio k_n/k_s of normal stiffness to shear stiffness. For the contact bond model, the macro elastic modulus E was mainly determined by the ratio of particle contact stiffness to stiffness. When the particle stiffness ratio was constant, the micro parameter E_c and macro elastic modulus presented linear correlation (Rahman et al., 2017). In the numerical triaxial test, the particle contact stiffness E_c needed to be continuously adjusted until the elastic modulus of numerical model was consistent with the indoor large-scale triaxial test, and the E_c at this time was the micro parameter determined. The stiffness ratio k_n/k_s was mainly determined by the Poisson's ratio, the greater the Poisson's ratio was, the greater the stiffness ratio was;

(2) Determination of particle micro parameter friction. The friction coefficient had a great effect on the peak intensity of the model, and little impact on the elastic modulus of the material, by adjusting the friction coefficient until it was similar to the peak intensity of the indoor large-scale triaxial test, the friction coefficient can be determined;

(3) Determination of particle bond strengths σ and τ . The particle bond strength was mainly related to the peak intensity of the model, when the ratio $\sigma_{mean}/\sigma_{dev}$ of bond strength average to unbalanced load value was constant, the peak intensity of the model was proportional to the bond strength average.

Table 2. Microparameters of the discrete element of sandy pebble soil of different gradation.

Particle gradation	Macromechanical properties of sandy pebble soil		Micro parameters of discrete element of sandy pebble soil					
	Internal friction angle φ	Cohesion c /kPa	Contact modulus E_c (N/m)	Porosity	Particle stiffness ratio	Friction coefficient	Bond stiffness (N/m)	Bond stiffness ratio
Gradation 1	43.3	118.2	2.00E+07	0.2	1.5	0.3	4.00E+04	4
Gradation 2	41.8	114.9	2.00E+07	0.2	1.5	0.2	4.00E+04	5
Gradation 3	39.4	99.6	1.70E+07	0.18	1.5	0.25	3.00E+04	3
Gradation 4	37.6	97.8	1.50E+07	0.15	1.5	0.2	3.00E+04	3

ANALYSIS OF RESEARCH RESULTS

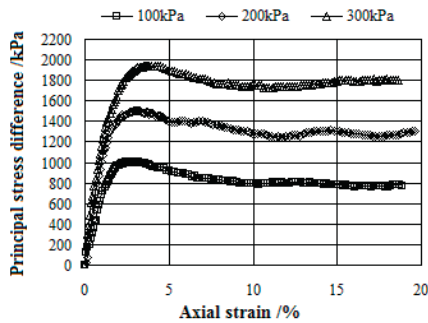
The stress peak of traditional the triaxial test model of particulate matter under different confining pressure was slightly lower than that of the indoor test, and Potyondy and Hoek et al. (1998) believed that by changing the particle micro parameters to overcome this disadvantage had little effect. In this numerical triaxial test, clump unit was introduced, which efficiently solved this problem. Clump unit was a large particle composed of a plurality of balls cemented together simulating the particles of irregular shape, which can realize the irregularity of sandy pebble soil, increase the occlusion and embedding among particles, thus appropriately improving the shear strength of the numerical triaxial test.

ANALYSIS OF MACROMECHANICAL PROPERTIES OF SANDY PEBBLE SOIL

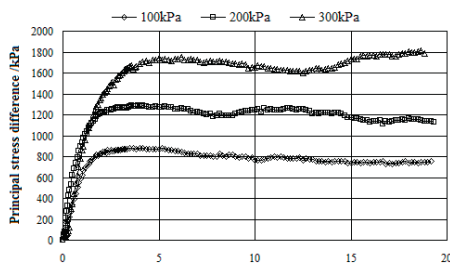
STRESS-STRAIN CHARACTERISTICS OF SANDY PEBBLE SOIL OF DIFFERENT COARSE-GRAINED CONTENT

From Figure 5, it can be seen that the stress-strain curve of sandy pebble soil exhibited strain softening with the increase of coarse-grained content, this was because, with the increase of coarse-grained content, particles became overhead, formed large pores, and lacked fine particles to fill, resulting in the decrease of stress with the increase of strain (Zhang et al., 2013; Basarian and

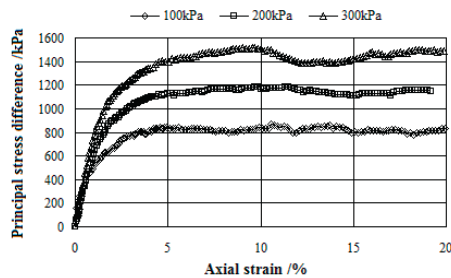
Tahir, 2017). From Figs. 6 and 7, it can be seen that when the confining pressure was the same, the stress maximum increased and the strain when the peak was reached decreased gradually with the increase of coarse-grained content.



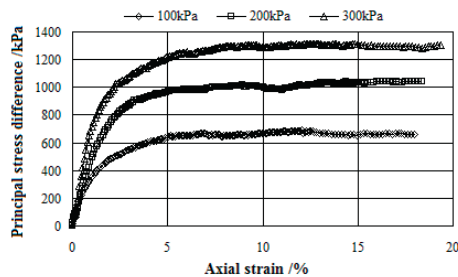
(a) Stress-strain curve of sandy pebble soil of gradation 1.



(b) Stress-strain curve of sandy pebble soil of gradation 2.



(c) Stress-strain curve of sandy pebble soil of gradation 3.



(d) Stress-strain curve of sandy pebble soil of gradation 4.

Figure 5. Stress-strain curve of sandy pebble soil of different coarse-grained content.

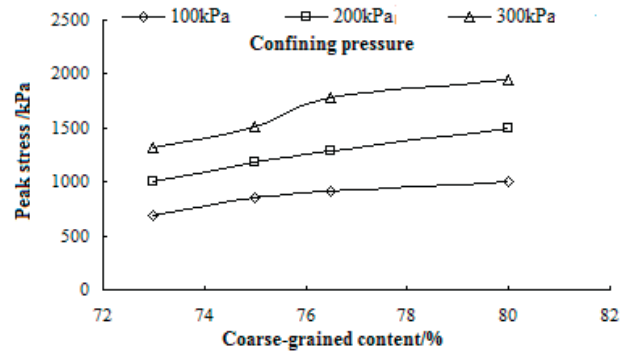


Figure 6. Peak stress of sandy pebble soil of different coarse-grained content.

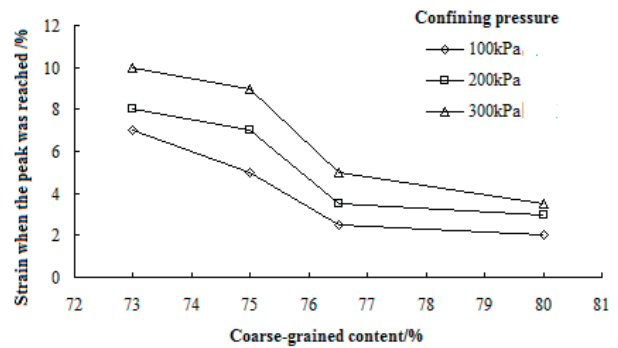


Figure 7. Peak strain of sandy pebble soil of different coarse-grained content.

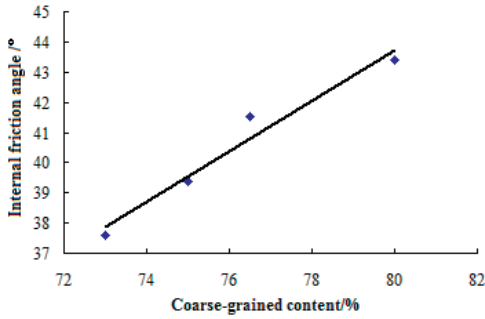
ANALYSIS OF MACROMECHANICAL PARAMETERS OF SANDY PEBBLE SOIL

From Figure 8, it can be seen that the internal friction angle and cohesion of sandy pebble soil linearly increased with the increase of coarse-grained content, but with different increasing magnitude, by fitting, the relationship between mechanical parameters and coarse-grained content of sandy pebble soil was obtained as follows:

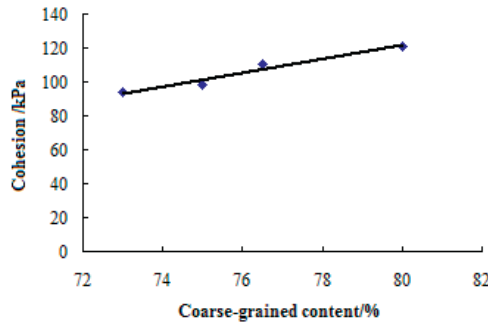
$$\varphi = 0.84P_5 - 23.39 \tag{1}$$

$$c = 4.13P_5 - 208.8 \tag{2}$$

Where: φ —internal friction angle, (°); c — cohesion, (kPa) ,



(a) Relationship between internal friction angle and coarse-grained content.



(b) Relationship between cohesion and coarse-grained content.

Figure 8. Relationship between internal friction angle and cohesion and coarse-grained content.

P5—mass percentage of particles with particle size of greater than 5 mm

ANALYSIS OF MICROMECHANICAL PROPERTIES OF SANDY PEBBLE SOIL

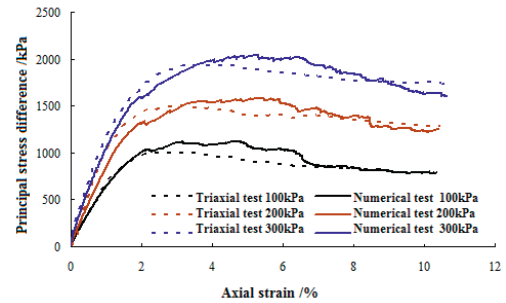
Based on the results of the indoor large-scale triaxial test of coarse-grained soil, the optimal micro parameters of sandy pebble soil of different coarse-grained content were obtained by using the approximation method of parameters of particle numerical triaxial test.

STRESS-STRAIN CURVE OF NUMERICAL TRIAXIAL TEST OF SANDY PEBBLE SOIL OF DIFFERENT COARSE-GRAINED CONTENT

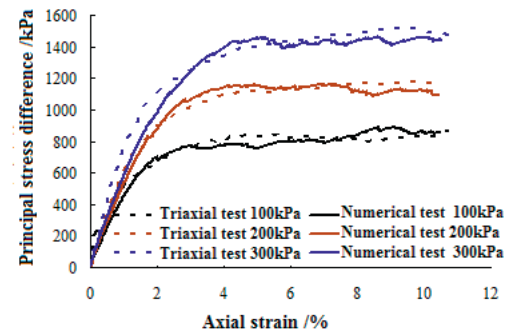
As can be seen from Figure 9, the stress-strain curve of the discrete element numerical triaxial test was in substantial agreement with that of the indoor test, and the micro parameters of the discrete element determined by this method can well realize the macromechanical properties of sandy pebble soil.

When the triaxial confining pressure was the same, the stress peak increased, but with different initial modulus, this was primarily because the basic unit of discrete element was the rigid sphere, and the occlusion of particles was weak, and the particles were arranged in single form, even if the contact bond was selected to improve the cohesion of particles (Bilal et al., 2017). However, with the increase of axial load, the bond fracture led to strength characteristics of sandy

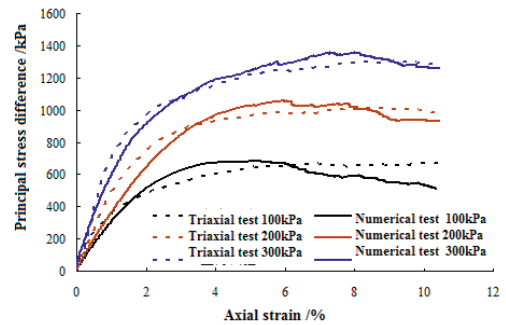
pebble soil different from the actual strength characteristics of sandy pebble soil, which also needed to be improved and further optimized.



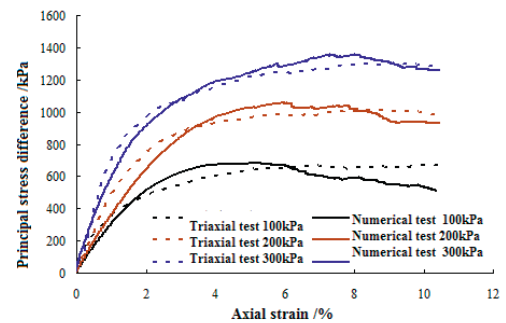
(a) Stress-strain curve of sandy pebble soil of gradation 1



(b) Stress-strain curve of sandy pebble soil of gradation 2



(c) Stress-strain curve of sandy pebble soil of gradation 3



(d) Stress-strain curve of sandy pebble soil of gradation 4

Figure 9. Stress-strain curve of sandy pebble soil of different coarse-grained content in the real and numerical triaxial test.

MICROMECHANICAL PARAMETERS OF DISCRETE ELEMENT OF SANDY PEBBLE SOIL

By checking calculation, the micro parameters of discrete element able to reflect the macromechanical properties of sandy pebble soil of different coarse-grained content were obtained, the determination of the micro parameters of the discrete element provided the basic data for the numerical analysis and design of sandy pebble soil. The specific micro parameters of the discrete element are shown in Table 2.

Table 2 showed that the micromechanical parameters of sandy pebble soil were mainly determined by contact modulus, porosity, friction coefficient, bond stiffness and bond stiffness ratio. The more the coarse-grained content was, the larger the contact modulus and friction coefficient were, and the higher the internal friction angle and cohesion of sandy pebble soil were, but the other micro parameters showed little change (Nema et al., 2017). Therefore, the fact that the strength of sandy pebble soil increased with the increase of coarse-grained content was caused by particle stiffness and friction coefficient.

ANALYSIS OF DISPLACEMENT OF SANDY PEBBLE SOIL PARTICLES

The displacement of sandy pebble soil particles in the triaxial compression process was analyzed, to reveal the micromovement of sandy pebble soil particles in the triaxial compression process. Because the coarse-grained content of sandy pebble soils of four different gradations had little difference, so their particle displacement fields showed no difference (Andik and Sarang, 2017). When the sandy pebble soil of gradation one was selected, the displacement was analyzed with particle displacement field of 100 kPa of confining pressure as the representative, and the microdisplacement diagram of particles under different strain is shown in Figure 11. From the beginning of compression shear, due to movement of upper and lower loading plates, the particles in the upper part of the model moved downward, and the particles in the lower part of the model moved upward. As the loading progressed, the particles in the middle of the model moved outward due to pressure, resulting in the dilatancy of specimens in the middle of the model, which was the same as the failure mode (Figure 10) of samples in the indoor test.



Figure 10. Failure appearance of specimens.

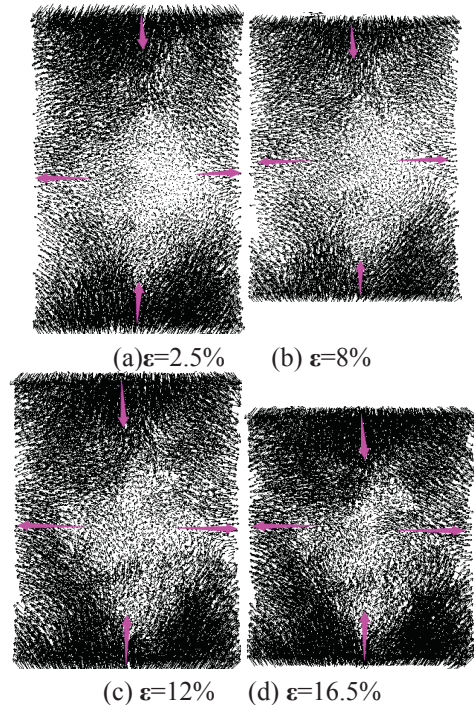


Figure 11. Displacement field of particles.

CONCLUSION

In this paper, the macromechanical parameters of sandy pebble soil of different coarse-grained content were obtained using the indoor large-scale triaxial test of sandy pebble soil, and the discrete element numerical triaxial model of sandy pebble soil was established. By checking calculation, the micromechanical parameters of sandy pebble soils of four different gradations were obtained, and the effect of coarse-grained content on the mechanical properties of sandy pebble soil was analyzed from the macro and micro points of view, and the following conclusions were obtained:

The stress-strain curve of sandy pebble soil exhibited strain softening with the rise of coarse-grained content; when the confining pressure was the same, the stress peak increased gradually with the increase of coarse-grained content;

The shear strength of sandy pebble soil increased steadily with the increase of coarse-grained content, and the internal friction angle and cohesion of sandy pebble soil linearly increased with the rise of coarse-grained content;

Based on the micromechanical parameters of sandy pebble soil in the triaxial test, the micromechanical parameters of discrete element of sandy pebble soil of four different gradations were obtained using the comparative data method of numerical triaxial test, providing the theoretical support for the fine research on stability of sandy pebble soil;

The increase of the coarse-grained content of sandy pebble soil resulted in increased contact stiffness and friction coefficient of micro parameters of discrete element, thus increasing the strength of sandy pebble soil;

The dilatancy of specimens was caused by the particles in the upper and lower parts of the model moving to the middle and the particles in the middle of the model moving outward, and the numerical triaxial test can better reproduce the triaxial compression process.

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REFERENCES

- Ahmad, N., Hussain, T., Awan, A.N., Sattar, A., Arslan, C., Tusief, M.Q., Mariam, Z. (2017). Efficient and Eco-friendly Management of biodegradable Municipal Solid Waste (MSW) using naturally aerated Windrow Composting Technique in District Lahore Pakistan. *Earth Science Pakistan*, 1(1), 01-04.
- Andik, B., Sarang, A. (2017). Daylighting Buried Rivers And Streams In Tehran. *Water Conservation And Management*, 1(2), 01-04.
- Basarian, M.S., Tahir, S.H. (2017). Groundwater Prospecting Using Geoelectrical Method At Kg Gana, Kota Marudu, Sabah. *Earth Science Malaysia*, 1(2), 07-09.
- Bilal, A., Yasin, M., Ali, A. (2017). The Geology And Structure Of Neogene Rocks In Dadyal And Adjacent Areas, In The Sub-Himalayas, Azad Jammu And Kashmir, Pakistan. *Earth Science Malaysia*, 1(1), 15-20.
- Cundall, P. A., & Strack, O. D. L. (1979). A discrete numerical model for granular assemblies. *Geotechnique*, 29(1), 47-65.
- Geng, L., Huang, Z., & Miao, Y. (2011). Meso-mechanics Simulation Triaxial Test of Coarse-grained Soil. *Journal of Civil Engineering and Management*, 28(4): 24-29.
- Guo, Q. (1998). *The engineering properties and application of coarse grained soil*. Zhengzhou: Yellow River Water Conservancy Press.
- Hejazi, S.M., Lotfi, F., Fashandi, H., Alirezazadeh, A. (2017). Serishm: An Eco-Friendly and Biodegradable Flame Retardant for Fabrics. *Environment & Ecosystem Science*, 1(2), 05-08.
- Hoek, E., & Brown, E. T. (1998). Practical estimates of rock mass strength. *International Journal of Rock Mechanics and Mining Sciences*, 34(8), 1165-1186.
- Kudus, K., Grasian, I., Madasamy, S., John, A. (2017). Immunomodulatory Effect of Alginic Acid from Brown Seaweed Sargassum Wightii on Disease Resistance in Penaeus Monodon. *Journal Clean WAS*, 1(1), 26-29.
- Nema, A., Yadav, K.D., Christian, R.A. (2017). Effect of Retention Time On Primary Media For Greywater Treatment. *Water Conservation and Management*, 1(1), 01-03.
- OmaraShahestan, M.J., OmaraShastani, S. (2017). Evaluating Environmental Considerations with Checklist And Delphi Methods, Case Study: Suran City, Iran. *Environment & Ecosystem Science*, 1(2), 01-04.
- Qi, Y., Tang, X., & Li, X. (2015). Stress-induced anisotropy of coarse-grained soil by true triaxial tests based on PFC. *Chinese Journal of Geotechnical Engineering*, 37(12), 2292-2300.
- Rahman, N.A., Tarmudi, Z., Rosdy, M., Muhiddin, F.A. (2017). Flood Mitigation Measures Using Intuitionistic Fuzzy Dematel Method. *Malaysian Journal Geosciences*, 1(2), 01-05.
- Ridzuan, A.A., Zahar, U.A.U., Noor, N.A.M. (2017). Association of Evacuation Dimensions towards Risk Perception of the Malaysian students who studied at Jakarta, Medan, and Aceh in Indonesia. *Malaysian Journal Geosciences*, 1(1), 07-12.
- Si, H. (1990). On the Mechanical Properties of Cohesionless Sand Cobbles and Rockfill Materials. *Chinese Journal of Geotechnical Engineering*, 12(6), 32-41.
- Wei, H., Wang, R., & Hu, M. (2008). Strength behavior of gravelly soil with different coarse-grained contents in Jiangjiagou ravine. *Rock and Soil Mechanics*, 29(1), 48-51.
- Wun, W.L., Chua, G.K., Chin, S.Y. (2017). Effect of Palm Oil Mill Effluent (POME) Treatment By Activated Sludge. *Journal Clean WAS*, 1(2), 06-09.
- Xu, X., Wei, H., & Meng, Q. (2013). DEM Simulation on Effect of Coarse Gravel Content to Direct Shear Strength and Deformation Characteristics of Coarse-grained Soil. *Journal of Engineering Geology*, 21(2), 311-316.
- Yasin, H., Usman, M., Rashid, H., Nasir, D.R., Randhawa, D.I.A. (2017). Alternative Approaches for Solid Waste Management: A Case Study in Faisalabad Pakistan. *Earth Sciences Pakistan*, 1(2), 07-09.
- Yasin, M. (2017). Diagenesis of Miocene Sandstone in the District Sudunhoti and Poonch, Azad Jammu and Kashmir, Pakistan. *Pakistan Journal of Geology*, 1(1), 05-07.
- Yasin, M., Shahzad, A., Abbasi, N., Ijaz, U., Khattak, Z. (2017). The Use of Stratigraphic Section In Recording Quaternary Environment For The Fluvial Depositional Environment – A Worked Example In District Poonch, Azad Jammu And Kashmir, Pakistan. *Pakistan Journal of Geology*, 1(2), 01-02.
- Zhang, C., Zhan, X., & Yang, C. (2013). Mesoscopic simulation of strength and deformation characteristics of coarse grained materials. *Rock and Soil Mechanics*, 34(7), 2077-2083.